#### ABN 31 105 704 078

13 Brock Street, Thomastown Victoria 3074 (P) +61 3 9464 4617 (F) +61 3 9464 4618



# GEOTECHNICAL SITE CLASSIFICATION LOT 311 FIVE FARMS ESTATE STAGE 3, CLYDE NORTH

Prepared for Frasers Property Australia c/- Beveridge Williams Pty Ltd

Report Reference: G4589.3

Date: 17 May 2022

#### ABN 31 105 704 078

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# **PROJECT DETAILS**

Project Reference	G4589.3	Rev	AK
Project Title	Five Farms Estate Stage 3		
Project Location	Clyde North	State	VIC
Date	17 May 2022		

# **CLIENT DETAILS**

Prepared For (Client)	Frasers Property Australia		
Prepared For (Facilitator)	Beveridge Williams Pty Ltd		
Client Address	Level 9, 484 St Kilda Road	Suburb	Melbourne

# **DISTRIBUTION**

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One (1) Electronic Copy	Beveridge Williams Pty Ltd

This document presents the results of the site classification conducted for the aforementioned project and is detailed for the sole use of the intended recipient. Should you have any questions related to this report please do not hesitate to contact the undersigned.

Author:

Gee Singh, MIEAust (NER) Senior Geotechnical Engineer

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#### 1. INTRODUCTION

This report presents the results of the geotechnical site classification investigation carried out by Ground Science for the Five Farms Estate, Stage 3 located in Clyde North, Victoria (the site). This report presents the results for:

#### Lot 311

The scope of works detailed herein was commissioned by Beveridge Williams Pty Ltd (facilitator) on behalf of Frasers Property Australia (Principal).

#### 2. PROJECT BACKGROUND

The Five Farms residential development included the construction of building platforms as part of the bulk earthworks phase of the project, which primarily involved the placement of controlled fill and site cuts. Controlled fill was placed and compacted to Level 1 procedures in general accordance with AS3798 (2007) 'Guidelines on Earthworks for Residential and Commercial Developments'. Ground Science were the nominated Geotechnical Inspection and Testing Authority for the Level 1 filling works carried out within this stage (report reference: GS5860.2 AA dated 19th April 2022).

The professional advice provided in this report is based on the information provided at the time of the report preparation and may not be valid if changes are made to the site, the development proposal, or the construction methods. In the event of such changes, further advice should be sought from Ground Science.

#### 3. PROJECT AIMS

The aims of the investigation were as follows:

- To assess the subsurface conditions at the site relevant to the proposed development through a desktop regional geological study.
- To recover soil samples for laboratory analysis.
- To log the soils as per the guidelines presented in AS1726 (2017) 'Geotechnical Site Investigations'.
- To classify the site in accordance with AS2870 (2011) 'Residential Slabs and Footings'.
- To calculate the characteristic surface movement (Y<sub>S</sub>) for the site.
- To provide advice on allowable bearing pressures and geotechnical parameters for the design of footing systems.
- To provide general construction advice.

### 4. SCOPE OF WORKS

The site classification investigation for Stages 2 & 3 was carried out simultaneously. The scope of works involved the drilling of 50 boreholes (BH1 – BH50) spatially distributed across the allotments using a truck mounted (GT10) drilling rig supplied and operated by Ground Science. The soils were logged and hand classified using visual tactile methods (AS1726) by a qualified geotechnical engineer from this office. Representative soil samples were recovered from the controlled fill & residual soil deposits for a total of 12 Shrink/Swell Index (Iss) tests. Laboratory testing was carried out in Ground Science's NATA accredited testing facility in Thomastown, Victoria.

Borehole locations are shown on the site plan in **Appendix A**. Engineering borehole log sheets for this allotment and surrounding allotments are presented in **Appendix B**. Laboratory test reports are presented in **Appendix C**.



#### 5. RESULTS

#### 5.1 REGIONAL GEOLOGICAL CONDITIONS

The Geological Survey of Victoria map sheets indicate the site is underlain by Miocene to Pliocene aged 'Red Bluff Sandstone' deposits, with Pleistocene to Holocene aged alluvial deposits indicated towards exist to the west and south/west. This assessment excludes controlled (engineered) fill noted to have been placed on the allotments, however the controlled fill materials are noted to have been generally derived from blending onsite soils or imported fill materials from local source sites of relatively similar geological characteristics.

#### 5.2 SURFACE CONDITIONS

The site is located west of Pound Road and southeast of the Hardys Road / future Bells Road intersection. At the time of our investigation, the site was observed to be generally flat with fair drainage conditions noted. The surface of the allotments was dry and generally comprised barren soil. The site was generally trafficable to a 4WD mounted drilling rig. No trees were observed to be located within close proximity to the building envelopes.

#### 5.3 SUBSURFACE CONDITIONS

The subsurface soil profile encountered during the field investigation is considered to be generally consistent with our expectations of the site. The subsurface soils generally comprised **controlled (engineered) fill** recovered as Sandy CLAY, Silty CLAY or Silty/Clayey SAND overlying **naturally occurring** inferred 'Red Bluff Sandstone' residual soils comprising Silty/Sandy CLAY. The controlled fill and residual soil deposits were generally medium dense to dense, stiff to very stiff, and moisture close to the plastic limit.

#### 5.4 GROUNDWATER

Groundwater was not encountered during the borehole drilling. The Visualising Victoria's Groundwater dataset indicates the regional groundwater table is less than 5.0mbgl. During wet seasons or following torrential rainfalls, there is a possibility for a perched water table to develop in the area. This should be carefully considered during the construction stage, especially when footing excavations are left exposed or prior to topsoil application.

#### 5.5 LABORATORY TESTING

A summary of the laboratory test results are presented in Table 1:

**Table 1: Laboratory Test Results Summary** 

Sample #	Borehole	Depth (m)	Geological Origin	Shrink Swell Index Iss (%)
S1	BH1	1.0 – 1.3	Residual Soils	3.8
S2	BH6	0.8 – 1.0	Controlled Fill	0.5
S3	BH10	1.5 – 1.8	Residual Soils	1.6
S5	BH16	0.6 – 0.9	Residual Soils	0.9
S6	BH19	1.5 – 1.7	Residual Soils	1.6
S7	BH22	1.0 – 1.3	Controlled Fill	3.5
S8	BH26	0.5 – 0.8	Controlled Fill	1.3
S10	BH32	1.5 – 1.7	Residual Soils	2.8
S12	BH41	0.5 – 0.8	Controlled Fill	0.7
S13	BH44	1.5 – 1.7	Residual Soils	2.9
S14	BH46	0.7 – 1.0	Residual Soils	0.4
S15	BH50	1.2 – 1.5	Residual Soils	1.3



#### 6. DISCUSSION & RECOMMENDATIONS

#### 6.1 AS2870-2011 SITE CLASSIFICATION

The site has been classified in general accordance with the guidelines presented in AS2870 (2011) 'Residential Slabs and Footings'. The following site characteristics were adopted in the site classification assessment:

**Table 2: Site Characteristics** 

Climatic Zone	2 (Figure D1 of AS2870-2011)
Soil Profile Group	Group 3 (Table D1 of AS2870-2011)
Depth of Soil Suction Change (Hs)	1.8m

Based on the results of the geotechnical investigation, the geological setting and the guidelines presented in AS2870 (2011), the site has been classified as **Class H1**, with an assessed characteristic surface movement (y<sub>s</sub>) of between 40mm and 60mm.

According to the Building Code of Australia (BCA), the above classification is only applicable for Class 1 to 10a building types. For other building types/loads, this classification should only be used as a guide. It is recommended that precautions be taken to control moisture variations within the founding soils given the variable reactivity of subsurface soils, as follows:

- Restrict tree planting in the vicinity of the building. AS2870-2011 advises that trees be planted no closer
  to the building than a distance equal to 1.0 times their mature height on Class H1 sites. This distance
  should be increased where rows or groups of trees are involved.
- Provide paving to the edge of the building to limit soil moisture variations due to seasonal wetting and drying. The paved surface should be graded away from the building such that run-off drains away, and water cannot pond against the building.
- Service trenches, particularly plumbing and drainage, should be avoided beneath buildings. Where
  service trenches are to pass beneath or close to the building, they should be backfilled with a low
  permeability material, such as compacted clay, to prevent the ingress of water. The use of porous backfill
  materials should be avoided.
- Any leaking or damaged underground services should be repaired promptly.
- During construction, footing excavations should not be left exposed to the weather for extended periods.
   Water should not be allowed to pond in these areas, nor should it be left unprotected to dry and crack in the sun.

#### 6.2 FOOTING DESIGN

The use of shallow / spread footings suitably embedded within the controlled fill or naturally occurring soils is considered suitable for this site. Footings shall be proportioned to an allowable bearing pressure of 100kPa, under stiff/medium dense and dry to damp conditions (or better).

It should be noted that construction during wet/winter periods may experience a reduced bearing pressure, particularly if left exposed for periods of time. Where required, a reassessment of the applicable bearing pressures may be undertaken. Footings should not be founded within any fill, unless the fill has been placed as controlled fill in accordance with AS3798 (2007) 'Guidelines for Earthworks on Residential and Commercial Developments' if applicable.



#### 7. GENERAL RECOMMENDATIONS

#### 7.1 FOOTINGS

- It is recommended that all footing excavations be inspected by a geotechnical engineer from this office to confirm that the founding conditions are consistent with design recommendations. The footing size and the founding level may need to be adjusted if the required founding material is not encountered at the design founding level.
- To reduce soil moisture variations near the footings, the builder should compact clean soil (without rubble or organic matter) around the footings to reduce potential water ingress around the footings.
- To reduce, but not eliminate, the possibility of damage to the footing, tree planting should be restricted as indicated earlier in this report.
- Good drainage is important to footing performance. The Builder should prevent water accumulation near the
  building footings (even during construction). It is recommended that sufficient ground clearance be created to
  accommodate paving which slopes a minimum of 1:20 away from the building. This slope should be achieved
  by excavation and not by building up loose fill around the footings.
- The roof water should be diverted away from the footing as soon as the roof is constructed by using temporary
  pipes, if necessary. The surface water should also be provided by constructing surface gutters or grading the
  surface to divert the water away from the footing.
- During wet conditions, machinery traffic may disturb the subgrade soils and should be avoided in the area of the building
- Any proposed footings which are close to an easement, underground service trenches, and/or other
  excavations, (including those in adjoining properties) should be founded below a line projected up at 45° to the
  horizontal (for firm/stiff Clay) and measured from the nearest base of the easement excavations.
- Avoid excavations close to footings since those founded on sandy soils can experience settlements while those founded in clayey soils can also move due to the shrinking and swelling of the clay. Plumbers and drainers should follow all the recommendations made in AS 2870-2011 and other appropriate codes with respect to drainage works.
- Protection of the footing system from moisture ingress or moisture loss after construction is the responsibility of the homeowner.

#### 7.2 DRAINAGE DESIGN REQUIREMENTS (AS2870-2011)

- It should be noted that the building and site drainage design, as well as height of the floor level above the finished ground level, may be affected by factors other than structural design requirements, such as below:
  - Run-off water and influence of local topography;
  - Possibility of flooding;
  - Effects of existing and post-construction landscaping;
  - Level of the legal point of stormwater discharge;
  - Plumbing and drainage requirements;
  - Minimum height from finished ground level to the damp-proof course level;
  - Termite management.



- Surface drainage shall be designed and constructed to avoid water ponding against or near the footing. The ground in the immediate vicinity of the perimeter footing, including the ground uphill from the slab on cut and fill sites shall be graded to fall 50mm minimum away from the footing over 1m and shaped to prevent ponding of water. Where the filling is placed adjacent to the building, the filling shall be compacted and graded to ensure drainage of water away from the building. The requirements of Clause 5.2.2 of AS2870 (2011) shall be applied to reduce the possibility of surface water entering living areas. Alternative drainage systems will be required on zero lot line construction. Any paving shall also be suitably sloped.
- The site classification as stated in this report shall be stated on any construction drawings. The selected footing system and any required site work and required site drainage shall be documented.

#### 7.3 SUBGRADE PREPARATION

- The subgrade should be stripped of all topsoil and soils containing significant organic matter.
- The exposed subgrade surface should be presented in a suitably moist condition and test rolled with several
  passes of an 8-10 tonne smooth drum roller. Any soft spots identified during test rolling should be removed by
  excavation and replaced with well-compacted suitable fill.
- Under no circumstances should any additional fill contain a significant amount of organic matter or be a mixture
  of greatly different particle sizes; e.g. it should not be a mixture of rock and soil, although less than 10% rock
  may be permitted.
- It is important that any fill be compacted close to its optimum moisture content during compaction.
- The compaction method and equipment should suit the fill material used and its degree of compaction should be tested and/or inspected by a suitably accredited NATA laboratory to meet the requirements of AS 3798-2007 "Guidelines on Earthworks for Commercial and Residential Developments".

#### 8. DISCLOSURE

This document is detailed for the sole use of the intended recipient(s) or its authorized representatives. Distribution of this report may be carried out at the Clients discretion and must be reproduced in full. Should you have any questions related to this report please do not hesitate to contact this office.



#### 9. LIMITATIONS

The advice provided in this document (as per our commission) is not designed or capable of identifying all soil conditions, (which can vary with products chosen). The advice given in this document is based on the assumption that the test results are representative of the overall soil conditions. However, it should be noted that actual conditions in some parts of the site might differ from those found. If further sampling/ testing reveals soil characteristics significantly different from those shown in our findings, Ground Science must be consulted.

The scope and the period of Ground Science services are described in the document and are subject to restrictions and limitations. Ground Science did not perform a complete assessment of all possible conditions or circumstances that may exist. If a service is not expressly indicated, do not assume it has been provided. If a matter is not addressed, do not assume that any determination has been made by Ground Science in regards to it.

Where data has been supplied by the client or a third party, it is assumed that the information is correct unless otherwise stated. No responsibility is accepted by Ground Science for incomplete or inaccurate data supplied by others.

It is recognized that the passage of time affects the information and assessment provided in this document. Ground Science's assessment is based on information that existed at the time of the preparation of this document. It is understood that the services provided allowed Ground Science to form no more than an opinion of the actual site conditions observed during sampling and observations of the site visit and cannot be used to assess the effects of any subsequent changes in the quality of the site, or its surroundings, or any laws or regulations.

Any drawings or figures presented in this report should be considered only as pictorial evidence of our work. Therefore, unless otherwise stated, any dimensions should not be used for accurate calculations or dimensioning.

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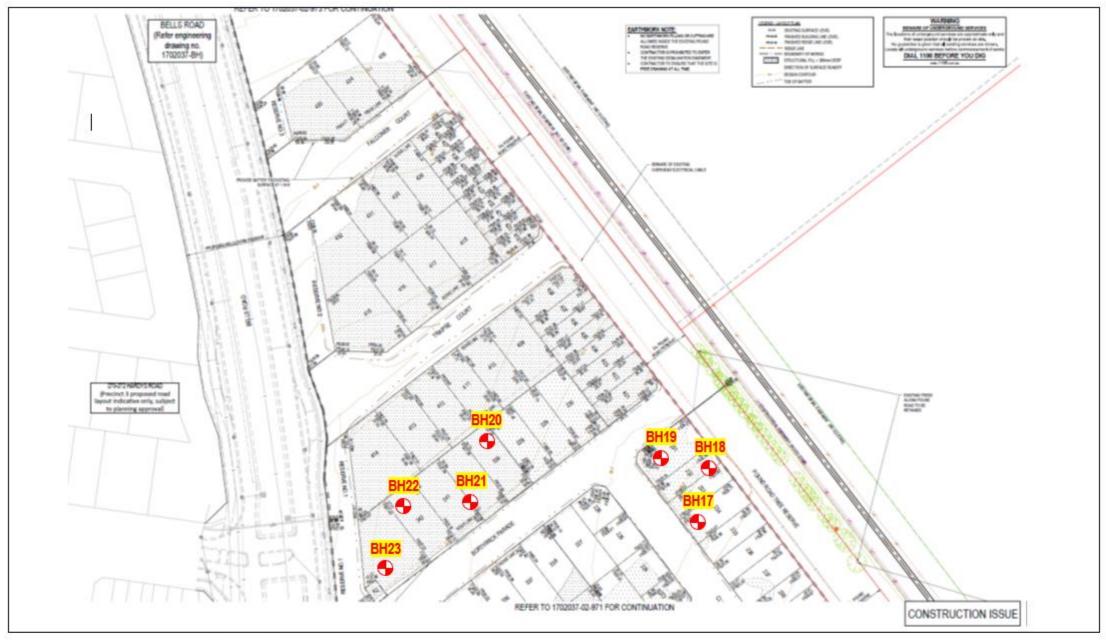


#### 10. REFERENCES

- Geological Survey of Victoria, Geological Map Sheets.
- AS2870 2011 Residential Slabs and Footings.
- AS3798 2007 Guidelines on Earthworks for Residential and Commercial Developments.
- AS1289 Testing of Soils for Engineering Purposes.
- AS1726 2017 Geotechnical Site Investigations.
- Ground Science, GS5860.2 AA 19 April 2022, Level 1 Inspection & Testing, Five Farms Stage 2.
- Ground Science, GS5860.3 AA 19 April 2022, Level 1 Inspection & Testing, Five Farms Stage 3.

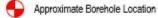
# APPENDIX A

Site & Test Location Plans



Rev	9	Drawn	Date	Checked	Scale	
				3		
0	Figure 1 of 2: Borehole Locations	cc	14.04.22	GS	NTS	

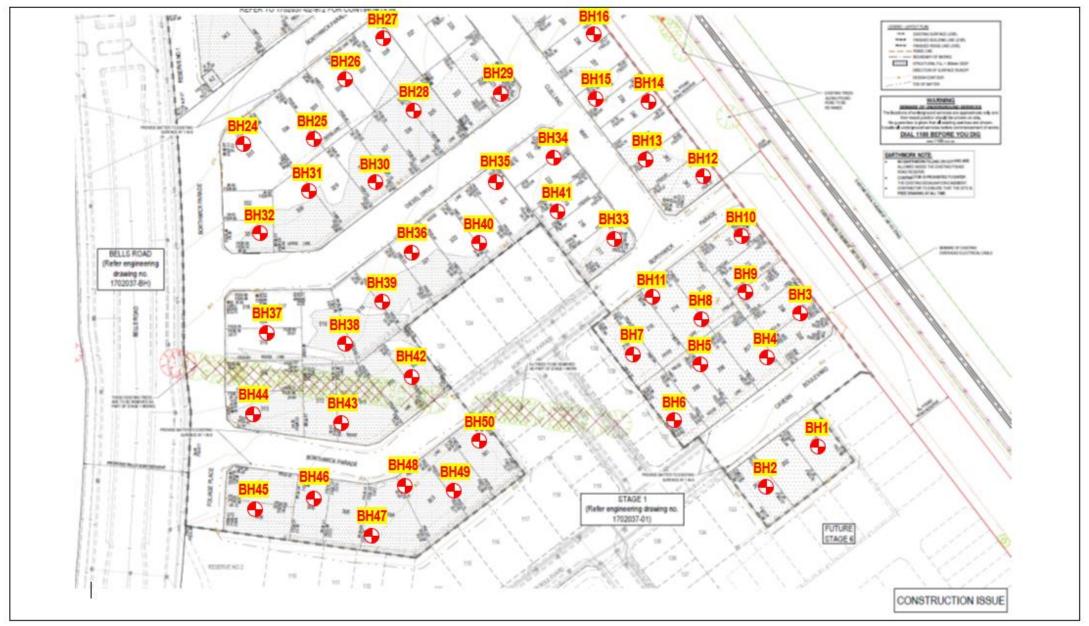
Legend



STAGE 2 & 3 BOREHOLE LOCATIONS FIVE FARMS ESTATE, CLYDE NORTH

> Prepared For: Frasers Property Australia Job No: G4589.2 & G4589.3





Rev		Drawn	Date	Checked	Scale	1
					5	0
					56 95	<del>571</del>
0	Figure 2 of 2: Proposed Boreholes	CC	14.04.22	GS	NTS	38

# Legend



STAGE 2 & 3 BOREHOLE LOCATIONS FIVE FARMS ESTATE, CLYDE NORTH

Prepared For: Frasers Property Australia

Job No: G4589.2 & G4589.3



# APPENDIX B

Engineering Borehole Logs

Ground	dScience	ENGINEERING BOREHOLE LOG	Borehole No	BH36
LOCATION: Clyde North TEST LOCATION: Refer to site plan, App DRILL METHOD: GT10 Drill Rig	ppendix A	EASTING: ND NORTHING: ND	TEST DATE: LOGGED BY: CHECKED BY: VANE SHEAR: INCLINATION:	13-Apr-22 NH GS N/A 90°
HOLE DIAMETER: 100mm  DRILLING	SAMPLING	FIELD MATERIAL DESCRIPTION	SURFACE RL:	ND
DEPTH (RI)	SAMPLE OR FIELD TEST RECOVERED GRAPHIC LOG USC SYMBOL	SOIL / ROCK MATERIAL DESCRIPTION	CONSISTENCY DENSITY MOISTURE	ADDITIONAL OBSERVATIONS
0.00		FilL: silty CLAY, medium to high plasticity, brown motifed brown-orange, with sand, gravel and brick fragments	St-VSt w<1	
1.0 1.0 1.60		sitly CLAY, high plasticity, brown-orange mottled red and grey, trace sand brown-orange mottled white and grey	VSt w≈	PL Inferred Red Bluff Sandstone Residual Soil
2.0 2.00  2.0 2.00  2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5	ISTENCY S Very Soft Soft Firm St Stiff St Very Stiff	VL Very Loose M Moist U50 Undisturbed		Groundwater Level UTP Unable to Penetrate

<b>U</b>		Gro	undSc	ien	ce	ENGINEERII	NG BOREHO	LE LOG	Borehole JOB No :		BH37 G4589.283	7	
CLIENT: PROJECT: LOCATION: TEST LOCAT		Five Farms Clyde North Refer to site	plan, Appendix A	tion - Stage	2&3	F107919 10			TEST DA' LOGGED CHECKEI VANE SH	BY: D BY: EAR:	13-Apr-22 NH GS N/A		
DRILL METHO HOLE DIAME	TER:	GT10 Drill 100mm			ı	EASTING: ND NORTHING: ND			SURFACE		90° ND		
2 PENERTRATION 2 RESISTANCE	WATER 4	DEPTH (metres) 56	SAMPLE OR FIELD TEST		GRAPHIC LOG	SOIL / RO	FIELD M	IATERIAL DESCRIPTION	CONSISTENCY DENSITY	MOISTURE	ADDIT	IONAL OBSERVATIONS	3
		0.30	ØF	<u> </u>		FILL: sity SAND, fine to coarse grained, angular to			MD - D	D-M		ontrolled Fill	
		1.20				silty CLAY, high plasticity, brown-orange mottled b	rown, red and grey, trace sand		VSt	w≈Pl	Inferred Red B	luff Sandstone Residual Soil	
PENETRATIO	3 3	2.00	CONSISTENCY			Borehole Terminated @ 2m	URE CONDITION	TEST NOTES					
no res	1 2 3	refusal	Vs S F St VSt	Very Soft Firm Stiff Very Hard		Fb         Friable         D           VL         Very Loose         M           L         Loose         W           MD         Medium Dense         w < P	Dry Moist Wet L Moist, dry of plastic limit 'L Moist, near plastic limit L Moist, near plastic limit L Moist, wet of plastic limit L Wet, near liquid limit L Wet, wet of liquid limit	PP Pocket Penetr U50 Undisturbed S U63 Undisturbed S D Disturbed San Bs Bulk Sample E Environmenta HSV Hand Shear V Cu Undrained Sh	ample 50mn ample 63mn nple I sample ane test	n n		undwater Level ble to Penetrate Sheet 1 of 1	

HAM\_LOG\_6.0 2019

•			Gro	undSo	ien:	ce	!	ENGINEERING BOREHOLE LOG	Borehole  JOB No :		BH38	
CLIENT: PROJECT: LOCATION: TEST LOCATI	D:	F C F	Five Farms E Clyde North Refer to site	erty Group Pty Ltd state Site Classifica olan, Appendix A	ition - Stage	e 2&3		EASTING: ND NORTHING: ND	TEST DA' LOGGED CHECKEI VANE SH	BY: D BY: EAR: TION:	13-Apr.22 NH GS N/A 90°	
HOLE DIAMET	TER: DRILI		100mm	SAMPLIN	IG			FIELD MATERIAL DESCRIPTION	SURFACE	E RL:	ND	
C PENERTRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL / ROCK MATERIAL DESCRIPTION	CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERVATION	s
		0.0				<b>***</b>	SM	FILL: silty SAND, fine to coarse grained, angular to subangular, brown, low plasticity silt, trace rootlets	MD - D	D-M	Controlled Fill	
		0.5	1.30					FILL: sitly CLAY, medium to high plasticity, brown mottled red and grey, with sand and gravel, trace brick fragments sitly CLAY, high plasticity, brown-orange mottled brown, red and grey, trace sand	St - VSt		Inferred Red Bluff Sandstone Residua Soil	
	Not Encountered	2.0	2.00					Borehole Terminated @ 2m				
>[	N 1 2		usal	CONSISTENCY VS S F St VSt H	Very Soft Firm Stiff Very Hard	Stiff		DENSITY    MOISTURE CONDITION   TEST NOTES	Sample 50mn Sample 63mn mple al sample Vane test	n n	▼ Groundwater Level UTP Unable to Penetrate	

<b>U</b>			Gro	undSc	ien	CE	•	ENGINEERING BOREHOLE LOG	Borehole		BH39	
CLIENT: PROJECT: LOCATION: TEST LOCAT DRILL METHO	DD:	1	Five Farms E Clyde North Refer to site	erty Group Pty Ltd Estate Site Classificat plan, Appendix A Rig	ion - Stag	e 2&3		EASTING: ND	TEST DA LOGGED CHECKE VANE SH INCLINA	BY: D BY: EAR: FION:	13-Apr-22 NH GS N/A 90°	
HOLE DIAME		LING	100mm	SAMPLIN	G	I		NORTHING: ND FIELD MATERIAL DESCRIPTION	SURFAC	E RL:	ND	
C PENERTRATION C RESISTANCE	WATER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL / ROCK MATERIAL DESCRIPTION	CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERVATIONS	:
	Ì	0.0				<b>***</b>	SM	FILL: silty SAND, fine to coarse grained, angular to subangular, brown, low plasticity silt, trace rootlets	MD - D	D-M	Controlled Fill	
		-										$\parallel \exists$
		-	0.20			<b>***</b>	CI-CH	FILL: silty CLAY, medium to high plasticity, brown mottled red and grey, with sand and gravel, trace brick fragments	St - VS	w > PL	-	
		-				888888 888888						
		0.5				<b>***</b>						
		-	0.60				СН	silty CLAY, high plasticity, brown-orange mottled brown, red and grey, trace sand	VSt	w≈PL	- Inferred Red Bluff Sandstone Residual	- 3
		_									Soil	
		-										=
		-										=
		1.0		S11	U63							
		-										
		-										
		-										=
		1.5										
	_	-										13
	unterec	-										
	Not Encountered	-										=
	ž	-										-
		2.0	2.00					Borehole Terminated @ 2m				
		-										
		-										
		-	1									
		2.5										
$ \  \  \  $		-	}									$\mid \exists$
$ \  \  \  $		_										
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		-	1									
$ \  \  \  $		3.0										
$ \  \  \  $		-	}									$\mid \exists$
		-	1									$\parallel \exists$
$ \  \  \  $		-										
		3.5										
PENETRATIO	N		•	CONSISTENCY	Very	Soft		DENSITY         MOISTURE CONDITION         TEST NOTES           Fb         Friable         D         Dry         PP         Pocket Pen	etrometer Test		▼ Groundwater Level	
	1 2	3 4		S	Soft			VL Very Loose M Moist U50 Undisturbed	Sample 50mr	n	UTP Unable to Penetrate	
F Firm St Stiff					Stiff			MD Medium Dense w < PL Moist, dry of plastic limit D Disturbed S		П		
VSt Very Stiff H Hard								D Dense w≈ PL Moist, near plastic limit Bs Bulk Sample  VD Very Dense w> PL Moist, wet of plastic limit E Environmen				
		re	fusal					w ≈ LL Wet, near liquid limit HSV Hand Shear				
HAM_LOG_6.0 2019								w > LE vvet, wet of liquid liftit   Ou Ondrained S	nicai Olienyth		Sheet 1 of 1	1

<b>U</b>			Gro	undSo	ien	ce	)	ENGINEERING BOREHOLE LOG	Borehole		BH40 G4589.2&3	
CLIENT: PROJECT: LOCATION: TEST LOCATI DRILL METHO	D:	F C	Five Farms E Clyde North Refer to site	erty Group Pty Ltd state Site Classifica plan, Appendix A	ation - Stag	e 2&3		EASTING: ND NORTHING: ND	TEST DA' LOGGED CHECKEI VANE SH	BY: D BY: EAR: FION:	13-Apr-22 NH GS N/A 90° ND	
HOLE DIAMET	ER: DRILL	ING	100mm	SAMPLIN	IG .			FIELD MATERIAL DESCRIPTION	SURFACI	E RL:	ND	
C PENERTRATION S RESISTANCE	WATER	DEPTH (metres)	DЕРТН (RL)	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL / ROCK MATERIAL DESCRIPTION	CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERVATIONS	
		0.0				<b>***</b>	SM	FILL: silty SAND, fine to coarse grained, angular to subangular, brown, low plasticity silt, trace rootlets	MD - D	D-M	Controlled Fill	-
			0.20			***	CI-CH	FILL: sitly CLAY, medium to high plasticity, brown-orange mottled red and grey, with sand and gravel, trace brick fregments	St - VSt	w > PL		
		0.5										
	Not Encountered	1.5	1.00				CH	silty CLAY, high plasticity, brown-crange motified brown, red and grey, trace sand	VSt	w≈PL	Inferred Red Bluff Sandstone Residual Soil	
		2.0	2.00					Borehole Terminated @ 2m				
no resi	N 1 2 istence	3 4	usal	CONSISTENCY VS S F St VSt H	Very Soft Firm Stiff Very Hard	Stiff		DENSITY         MOISTURE CONDITION         TEST NOTES           Fb         Friable         D         Dry         PP         Pocket Penetro           VL         Very Loose         M         Moist         U50         Undisturbed Se           L         Loose         W         Wet         U63         Undisturbed Se           MD         Medium Dense         w < PL Moist, dry of plastic limit	mple 50mr mple 63mr ble sample ne test	n n	▼ Groundwater Level UTP Unable to Penetrate  Sheet 1 of 1	

<b>U</b>			Gro	undSc	ien	ce	)	ENGINEERING BOREHOLE LOG  Borehole No. BH41  JOB No: G4589 2&3	
CLIENT: PROJECT: LOCATION: TEST LOCATION DRILL METHO	D:	F	Five Farms E Clyde North Refer to site GT10 Drill	erty Group Pty Ltd state Site Classificat plan, Appendix A Rig	tion - Stag	e 2&3		TEST DATE: 13-App-22	
HOLE DIAMET	ER: DRILI	LING	100mm	SAMPLIN	G	1		NORTHING:         ND           SURFACE RL:         ND   FIELD MATERIAL DESCRIPTION	
5 PENERTRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL / ROCK MATERIAL DESCRIPTION  SOIL / ROCK MATERIAL DESCRIPTION  ADDITIONAL OBSERVATION  SOIL / ROCK MATERIAL DESCRIPTION  ADDITIONAL OBSERVATION  ADDITIONAL OBSERVATION  SOIL / ROCK MATERIAL DESCRIPTION	NS
		0.0				*** ***	SM	FILL: silty SAND, fine to coarse grained, angular to subangular, brown, low plasticity silt, trace rootlets MD - D D - M Controlled Fill	
		- - - - -	0.20				CI-CH	FILL: silty CLAY, medium to high plasticity, brown-orange mottled red and grey, with sand and gravel, trace brick  St - VSt w > PL fragments	
		0.5		S12	U63				
		1.0	1.00				СН	sity CLAY, high plasticity, brown-orange mottled pale grey, trace sand  VSt	al
		1.5							
	Not Encountered								
П		2.0	2.00			<i>(</i> 3 <i>(</i> 2)		Borehole Terminated @ 2m	-
		2.5							
		- - - -							
		3.0							
		-   -   -   -							
PENETRATIO	N	3.5		CONSISTENCY		Ш		DENSITY MOISTURE CONDITION TEST NOTES	17
7	1 2	ref	usal	Vs S F St VSt H	Very Soft Firm Stiff Very Hard	Stiff		Fb Friable  VL Very Loose  M Moist  U50  Undisturbed Sample 50mm  UTP  Unable to Penetrate  U50  Undisturbed Sample 63mm  UTP  Unable to Penetrate  U63  Undisturbed Sample 63mm  U79  Unable to Penetrate  U63  Undisturbed Sample 63mm  U79  Unable to Penetrate  U63  Undisturbed Sample 63mm  D  Disturbed Sample  BB  Bulk Sample  VD  Very Dense  W > PL Moist, wet of plastic limit  W ≈ LL Wet, near liquid limit  W > LL Wet, wet of liquid limit  W > LL Wet, wet of liquid limit  W > LL Wet, wet of liquid limit  Sheet 10	

<b>U</b>		Gro	oundSo	ien	ce	!	ENGINEERING BOREHOLE LOG	Borehole		BH42 <sub>G4589.2&amp;3</sub>	
CLIENT: PROJECT: LOCATION: TEST LOCATI DRILL METHO	D:	Five Farms Clyde Nort Refer to sit	e plan, Appendix A	tion - Stag	e 2&3		EASTING: ND NORTHING: ND	TEST DA' LOGGED CHECKEI VANE SH	BY: D BY: IEAR: TION:	13-Apr-22 NH GS N/A 90° ND	
HOLE DIAMET	TER: DRILL	100mm ING	SAMPLIN	IG			FIELD MATERIAL DESCRIPTION	SURFACI	E RL:	ND	
C PENERTRATION S RESISTANCE	WATER	DEPTH (metres) DEPTH (RL)	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL / ROCK MATERIAL DESCRIPTION	CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERVATIONS	
		0.0			***	SM	FILL: silty SAND, fine to coarse grained, angular to subangular, brown, low plasticity silt, trace rootlets	MD - D	D-M	Controlled Fill	-
		0.20	_				FilL: sity CLAY, medium to high plasticity, brown mottled grey, with sand and gravel, trace brick fragments brown-orange motified red and grey	St - VSt	t w > PL		
		- - - - - - - - -									1111
	Not Encountered	1.60					sity CLAY, high plasticity, brown-orange mottled grey, trace sand  brown-orange mottled grey and red	VSt	w≈PL	Inferred Red Bluff Sandstone Residual Soil	
		20 2.00					Borehole Terminated @ 2m				- - - - - - - - - - - - - - - - - - -
PENETRATION		30	CONSISTENCY VS S F	Very Soft Firm			DENSITY         MOISTURE CONDITION         TEST NOTES           Fb         Friable         D         Dry         PP         Pocket Penetror           VL         Very Loose         M         Moist         U50         Undisturbed Sar           L         Loose         W         Wet         U63         Undisturbed Sar	nple 50mr	m	Groundwater Level UTP Unable to Penetrate	
no resi	istence	refusal	F St VSt H	Firm Stiff Very Hard	Stiff		L Losse W Wet U63 Undisturbed Sar W PL Moist, dry of plastic limit D Dense w = PL Moist, near plastic limit B B Bulk Sample  VD Very Dense w > PL Moist, near plastic limit w = LL Wet, near liquid limit W > LL Wet, wet of liquid limit Cu Undrained Shea	le ample ne test		Sheet 1 of 1	

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Ground Science				ien	ce	)	ENGINEERING BOREHOLE LOG	Borehole JOB No		BH43 64589.283		
CLIENT: PROJECT: LOCATION: TEST LOCATI		F C R	ive Farms E Clyde North Refer to site p	erty Group Pty Ltd state Site Classifica slan, Appendix A	ition - Staç	je 2&3		EASTING: ND	LOGGED CHECKE VANE SH	BY: DBY: HEAR:	13-Apr-22 NH GS N/A 90°	
HOLE DIAMET			GT10 Drill F 100mm	SAMPLIN	10			NORTHING: ND	SURFAC		ND	
2 PENERTRATION 8 RESISTANCE	TER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD FET TEST	RECOVERED	GRAPHIC LOG	JSC SYMBOL	FIELD MATERIAL DESCRIPTION  SOIL / ROCK MATERIAL DESCRIPTION	CONSISTENCY	MOISTURE	ADDITIONAL OBSERVATIONS	
	*   >	0.0		8 1	"	₩ ₩		FILL: sitly CLAY, medium to high plasticity, brown mottled grey and red, with sand and gravel, trace brick fragments		t w < PL	. Controlled Fill	T
			0.20							w > PL		
		0.5	0.50				CH	silty CLAY, high plasticity, brown mottled grey, trace sand	VSt	w≈PL	Inferred Red Bluff Sandstone Residual Soil	
	Not Encountered	1.5	1.60					brown-crange motifed grey and red				-
		2.0	2.00					Borehole Terminated @ 2m				
no resi	1 2	refu	usal	Vs S F St VSt H	Soft Firm Stiff	Stiff		DENSITY         MOISTURE CONDITION         TEST NOTES           Fb         Friable         D         Dry         PP         Pocket Penetr           VL         Very Loose         M         Moist         U50         Undisturbed Sx           L         Loose         W         Wet         U63         Undisturbed Sx           MD         Medium Dense         w < PL Moist, dry of plastic limit         Bs         Bulk Sample           VD         Very Dense         w > PL Moist, wer of plastic limit         E         Environmental two plants           W > LL Wet, near liquid limit         HSV         Hand Shear Vironmental two plants           W > LL Wet, wet of liquid limit         Cu         Undrained Shear Vironmental Cu	ample 50mi ample 63mi ple sample ane test	m m	▼ Groundwater Level UTP Unable to Penetrate  Sheet 1 of 1	

Ground Science					ien	ce	)	ENGINEERING BOREHOLE LOG			BH44		
CLIENT: PROJECT: LOCATION: TEST LOCATI		F C R	ive Farms E lyde North tefer to site p	erty Group Pty Ltd state Site Classifica olan, Appendix A	tion - Stag	e 2&3		EASTING: ND	LOGGED CHECKE VANE SH	BY: DBY: HEAR:	13-Apr-22 NH GS N/A 90°		
DRILL METHO HOLE DIAMET	ER:		GT10 Drill F 100mm		_	1		NORTHING: ND	SURFAC		ND		
2 PENERTRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD YS	RECOVERED	GRAPHIC LOG	USC SYMBOL	FIELD MATERIAL DESCRIPTION  SOIL / ROCK MATERIAL DESCRIPTION	CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERVATIONS		
		0.5	0.10					FilL: silty CLAY, medium to high plasticity, brown mottled grey and red, with sand and gravel, trace brick fragments as the state of th		w > PL w ≈ PL			
	Not Encountered	1.5	1.60	\$13	U63			pale grey mottled brown-orange and red					
		25	2.00					Borehole Terminated @ 2m					
no resi	1 2	refu	usal	CONSISTENCY VS S F St VSt H	Very Soft Firm Stiff Very Hard	Stiff		DENSITY         MOISTURE CONDITION         TEST NOTES           Fb         Friable         D         Dry         PP         Pocket Penetr           VL         Very Loose         M         Moist         U50         Undisturbed S           L         Loose         W         Wet         U63         Undisturbed S           MD         Medium Dense         w < PL Moist, dry of plastic limit	ample 50mi ample 63mi iple sample ane test	m m	▼ Groundwater Level UTP Unable to Penetrate  Sheet 1 of 1		

# APPENDIX C

Laboratory Test Reports

# **Material Test Report**

Report Number: G4589.2-1

Issue Number:

**Date Issued:** 02/05/2022

Client: Frasers Property Australia c/- Beveridge Williams

1 Glenferrie Road, Malvern VIC 3144

Project Number: G4589.2

Project Name: Five Farms Residential Development - Stage 2/3

Project Location: Clyde North Work Request: 8075

Report Number: G4589.2-1

**Dates Tested:** 14/04/2022 - 26/04/2022



Ground Science Pty Ltd Ground Science Laboratory

13 Brock Street Thomastown Victoria 3074

Phone: (03) 9464 4617

Email: pelin@groundscience.com.au Accredited for compliance with ISO/IEC 17025 - Testing



Approved Signatory: Pelin Erden gs-pelin

NATA Accredited Laboratory Number: 15055

Shrink Swell Index AS 1289 7.1.1 & 2.1.1					
Sample Number	4589.2-S1	4589.2-S2	4589.2-S3	4589.2-S5	4589.2-S6
Date Sampled	14/04/2022	14/04/2022	14/04/2022	14/04/2022	14/04/2022
Date Tested	19/04/2022	19/04/2022	19/04/2022	19/04/2022	20/04/2022
Material Source	**	**	**	**	**
Sample Location	BH1 (1.0 - 1.3m)	BH6 (0.8 - 1.0m)	BH10 (1.5 - 1.8m)	BH16 (0.6 - 0.9m)	BH19 (1.5 - 1.7m)
Inert Material Estimate (%)	0	10	5	5	0
Pocket Penetrometer before (kPa)	250	150	260	150	250
Pocket Penetrometer after (kPa)	135	80	200	100	125
Shrinkage Moisture Content (%)	31.7	17.6	18.1	15.9	18.6
Shrinkage (%)	6.3	0.9	1.9	1.6	2.5
Swell Moisture Content Before (%)	29.1	18.5	20.0	13.0	17.6
Swell Moisture Content After (%)	35.7	23.8	25.2	19.6	18.7
Swell (%)	1.1	-0.1	1.9	-0.0	0.9
Shrink Swell Index Iss (%)	3.8	0.5	1.6	0.9	1.6
Visual Description	silty CLAY, medium to high plasticity, mottled brown, grey	silty CLAY, low plasticity, mottled brown, grey, trace gravel, trace sand	Silty CLAY, medium to high plasticity, mottled brown, grey, trace gravel, trace sand	Sandy CLAY, low plasticity, brown, trace gravel	silty CLAY, medium plasticity, mottled brown, grey, orange, trace sand
Cracking	SC	MC	SC	MC	SC
Crumbling	No	No	No	No	No
Remarks	**	**	**	**	**

Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.

Cracking Terminology: UC Uncracked, SC Slightly Cracked, MC Moderately Cracked, HC Highly Cracked, FR Fragmented.

NATA Accreditation does not cover the performance of pocket penetrometer readings.

# **Material Test Report**

Report Number: G4589.2-1

Issue Number:

**Date Issued:** 02/05/2022

Client: Frasers Property Australia c/- Beveridge Williams

1 Glenferrie Road, Malvern VIC 3144

Project Number: G4589.2

Project Name: Five Farms Residential Development - Stage 2/3

Project Location: Clyde North Work Request: 8075

Report Number: G4589.2-1

**Dates Tested:** 14/04/2022 - 26/04/2022



Ground Science Pty Ltd Ground Science Laboratory

13 Brock Street Thomastown Victoria 3074 Phone: (03) 9464 4617

Email: pelin@groundscience.com.au

Accredited for compliance with ISO/IEC 17025 - Testing



Approved Signatory: Pelin Erden gs-pelin

NATA Accredited Laboratory Number: 15055

Shrink Swell Index AS 1289 7.1.1 & 2.1.1					
Sample Number	4589.2-S7	4589.2-S8	4589.2-S10	4589.2-S12	4589.2-S13
Date Sampled	14/04/2022	14/04/2022	14/04/2022	14/04/2022	14/04/2022
Date Tested	20/04/2022	20/04/2022	20/04/2022	21/04/2022	21/04/2022
Material Source	**	**	**	**	**
Sample Location	BH22 (1.0 - 1.3m)	BH26 (0.5 - 0.8m)	BH32 (1.5 - 1.7m)	BH41 (0.5 - 0.8m)	BH44 (1.5 - 1.7m)
Inert Material Estimate (%)	0	5	5	0	0
Pocket Penetrometer before (kPa)	200	200	300+	150	300+
Pocket Penetrometer after (kPa)	150	150	125	100	150
Shrinkage Moisture Content (%)	23.4	22.2	22.3	18.7	24.9
Shrinkage (%)	6.2	2.3	4.4	1.2	4.1
Swell Moisture Content Before (%)	23.0	24.2	19.8	16.7	25.6
Swell Moisture Content After (%)	22.2	28.1	23.6	25.9	29.7
Swell (%)	0.3	0.2	1.4	0.1	2.4
Shrink Swell Index Iss (%)	3.5	1.3	2.8	0.7	2.9
Visual Description	Silty CLAY, low to medium plasticity, mottled brown, grey, orange, trace sand	silty CLAY, low to medium plasticity, mottled brown, grey, orange, trace gravel, trace sand	silty CLAY, medium plasticity, mottled grey, orange, red, trace gravel, trace sand	sandy CLAY, low plasticity, brown	Silty CLAY, medium to high plasticity, mottled brown, grey, orange, trace sand
Cracking	SC	SC	SC	HC	SC
Crumbling	No	No	No	No	No
Remarks	**	**	**	**	**

Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.

Cracking Terminology: UC Uncracked, SC Slightly Cracked, MC Moderately Cracked, HC Highly Cracked, FR Fragmented.

NATA Accreditation does not cover the performance of pocket penetrometer readings.

# **Material Test Report**

Report Number: G4589.2-1

Issue Number:

**Date Issued:** 02/05/2022

Client: Frasers Property Australia c/- Beveridge Williams

1 Glenferrie Road, Malvern VIC 3144

Project Number: G4589.2

Project Name: Five Farms Residential Development - Stage 2/3

Project Location: Clyde North Work Request: 8075

Report Number: G4589.2-1

**Dates Tested:** 14/04/2022 - 26/04/2022



Ground Science Pty Ltd Ground Science Laboratory 13 Brock Street Thomastown Victoria 3074

Phone: (03) 9464 4617

Email: pelin@groundscience.com.au

Accredited for compliance with ISO/IEC 17025 - Testing



Approved Signatory: Pelin Erden gs-pelin

NATA Accredited Laboratory Number: 15055

Shrink Swell Index AS 1289 7.1.1 & 2.1.1				
Sample Number	4589.2-S14	4589.2-S15		
Date Sampled	14/04/2022	14/04/2022		
Date Tested	26/04/2022	26/04/2022		
Material Source	**	**		
Sample Location	BH46 (0.7 - 1.0m)	BH50 (1.2 - 1.5m)		
Inert Material Estimate (%)	10	5		
Pocket Penetrometer before (kPa)	120	160		
Pocket Penetrometer after (kPa)	110	150		
Shrinkage Moisture Content (%)	16.8	15.9		
Shrinkage (%)	0.8	2.1		
Swell Moisture Content Before (%)	15.3	15.0		
Swell Moisture Content After (%)	19.9	23.8		
Swell (%)	-0.1	0.4		
Shrink Swell Index Iss (%)	0.4	1.3		
Visual Description	silty CLAY, medium plasticity, brown, trace gravel, with sand	silty CLAY, medium plasticity, mottled brown, grey, orange, trace gravel, with sand		
Cracking	SC	SC		
Crumbling	No	**		
Remarks	**	**		

Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.

Cracking Terminology: UC Uncracked, SC Slightly Cracked, MC Moderately Cracked, HC Highly Cracked, FR Fragmented.

NATA Accreditation does not cover the performance of pocket penetrometer readings.